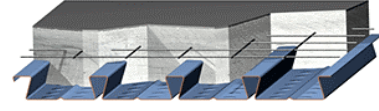


SECTION PROPERTIES  $F_y = 276 \text{ MPa}$

GAGE	Wd	$I_D$	Sp	Sn	Rbe			Rbi		
					51 mm	76 mm	102 mm	102 mm	127 mm	152 mm
22	10.90	555157	393153	383867	14.13	16.26	18.07	26.73	28.82	30.72
20	13.22	675466	493114	473586	20.11	23.06	25.54	37.95	40.82	43.43
18	17.48	897920	659170	661491	33.50	38.16	42.09	63.00	67.53	71.63
16	22.01	1136033	838335	843114	51.08	57.86	63.58	95.82	102.40	108.34



SIMPLE SPAN - MAXIMUM SUPERIMPOSED LSD LOADS, (kPa), NO STUDS ON BEAMS																	
h (Wc)		102 mm (216.72)				108 mm (231.47)				114 mm (246.22)				121 mm (260.97)			
Span (mm)	Load Combinations	GAGE															
		22	20	18	16	22	20	18	16	22	20	18	16	22	20	18	16
2430	$\alpha_D D + \alpha_L L$ (Strength)	13.55	18.76	23.94	23.94	14.78	19.40	23.94	23.94	16.04	19.90	23.94	17.32	20.26	23.94	23.94	
	D+L (Deflection)	13.55	18.76	19.15	19.15	14.78	19.15	19.15	19.15	16.04	19.15	19.15	17.32	19.15	19.15	19.15	
	L (Deflection)	13.55	18.76	19.15	19.15	14.78	19.15	19.15	19.15	16.04	19.15	19.15	17.32	19.15	19.15	19.15	
2740	$\alpha_D D + \alpha_L L$ (Strength)	10.45	11.80	19.54	23.90	11.42	14.80	20.30	23.94	12.40	16.07	20.93	23.94	13.39	17.36	21.43	23.94
	D+L (Deflection)	10.45	11.80	19.15	19.15	11.42	14.80	19.15	19.15	12.40	16.07	19.15	19.15	13.39	17.36	19.15	19.15
	L (Deflection)	10.45	11.80	19.15	19.15	11.42	14.80	19.15	19.15	12.40	16.07	19.15	19.15	13.39	17.36	19.15	19.15
3040	$\alpha_D D + \alpha_L L$ (Strength)	8.23	10.86	13.15	19.36	9.00	11.87	13.34	20.36	9.78	12.90	18.12	21.04	10.57	13.94	20.14	21.59
	D+L (Deflection)	8.23	10.86	13.15	19.15	9.00	11.87	13.34	19.15	9.78	12.90	18.12	19.15	10.57	13.94	19.15	19.15
	L (Deflection)	8.23	10.86	13.15	16.36	9.00	11.87	13.34	19.15	9.78	12.90	18.12	19.15	10.57	13.94	19.15	19.15
3350	$\alpha_D D + \alpha_L L$ (Strength)	6.57	8.85	11.31	13.75	7.20	9.68	12.74	13.89	7.83	10.53	14.28	15.55	8.47	11.39	15.90	17.33
	D+L (Deflection)	6.57	8.85	11.31	13.75	7.20	9.68	12.74	13.89	7.83	10.53	14.28	15.55	8.47	11.39	15.90	17.33
	L (Deflection)	6.57	8.85	11.19	12.29	7.20	9.68	12.74	13.89	7.83	10.53	14.28	15.55	8.47	11.39	15.90	17.33
3650	$\alpha_D D + \alpha_L L$ (Strength)	5.31	7.31	8.93	9.79	5.82	8.00	10.10	11.05	6.34	8.71	11.35	12.41	6.87	9.43	12.68	13.84
	D+L (Deflection)	5.31	7.31	8.93	9.79	5.82	8.00	10.10	11.05	6.34	8.71	11.35	12.41	6.87	9.43	12.68	13.84
	L (Deflection)	5.31	7.31	8.62	9.47	5.82	8.00	10.10	11.05	6.34	8.71	11.35	12.41	6.87	9.43	12.68	13.84
3960	$\alpha_D D + \alpha_L L$ (Strength)	4.32	5.97	7.08	7.80	4.74	6.54	8.04	8.84	5.17	7.13	9.07	9.97	5.61	7.72	10.17	11.16
	D+L (Deflection)	4.32	5.97	7.08	7.80	4.74	6.54	8.04	8.84	5.17	7.13	9.07	9.97	5.61	7.72	10.17	11.16
	L (Deflection)	4.32	5.97	6.78	7.45	4.74	6.54	7.98	8.76	5.17	7.13	9.07	9.97	5.61	7.72	10.17	11.16
4260	$\alpha_D D + \alpha_L L$ (Strength)	3.53	4.88	5.61	6.22	3.89	5.38	6.41	7.09	4.25	5.87	7.27	8.03	4.61	6.37	8.18	9.05
	D+L (Deflection)	3.53	4.88	5.61	6.22	3.89	5.38	6.41	7.09	4.25	5.87	7.27	8.03	4.61	6.37	8.18	9.05
	L (Deflection)	3.53	4.87	5.43	5.96	3.89	5.38	6.39	7.02	4.25	5.87	7.27	8.03	4.61	6.37	8.18	9.05
4570	$\alpha_D D + \alpha_L L$ (Strength)	2.88	3.80	4.42	4.95	3.18	4.39	5.09	5.68	3.48	4.86	5.81	6.46	3.43	5.28	6.58	7.33
	D+L (Deflection)	2.88	3.67	4.32	4.93	3.18	4.39	5.09	5.68	3.48	4.86	5.81	6.46	3.43	5.28	6.58	7.33
	L (Deflection)	2.88	3.67	4.32	4.85	3.18	4.39	5.09	5.68	3.48	4.86	5.81	6.46	3.43	5.28	6.58	7.33
4870	$\alpha_D D + \alpha_L L$ (Strength)	2.02	2.91	3.45	3.90	2.25	3.41	4.01	4.52	2.48	3.94	4.62	5.18	2.72	4.38	5.27	5.89
	D+L (Deflection)	2.02	2.91	3.15	3.65	2.25	3.36	3.98	4.52	2.48	3.94	4.62	5.18	2.72	4.38	5.27	5.89
	L (Deflection)	2.02	2.63	3.15	3.65	2.25	3.36	3.98	4.52	2.48	3.94	4.62	5.18	2.72	4.38	5.27	5.89
5180	$\alpha_D D + \alpha_L L$ (Strength)	1.56	2.18	2.65	3.04	1.74	2.59	3.12	3.56	1.93	3.04	3.63	4.12	2.13	3.52	4.18	4.74
	D+L (Deflection)	1.56	1.82	2.25	2.65	1.74	2.40	2.91	3.39	1.93	3.04	3.63	4.12	2.13	3.52	4.18	4.74
	L (Deflection)	1.56	1.82	2.25	2.65	1.74	2.40	2.91	3.39	1.93	3.04	3.63	4.12	2.13	3.52	4.18	4.74
5480	$\alpha_D D + \alpha_L L$ (Strength)	1.17	1.56	1.97	2.32	1.32	2.19	2.37	2.76	1.48	2.42	2.80	3.23	1.63	2.66	3.27	3.73
	D+L (Deflection)	0.96	1.18	1.53	1.86	1.32	1.65	2.07	2.46	1.48	2.19	2.68	3.14	1.63	2.66	3.27	3.73
	L (Deflection)	0.96	1.18	1.53	1.86	1.32	1.65	2.07	2.46	1.48	2.19	2.68	3.14	1.63	2.66	3.27	3.73
5790	$\alpha_D D + \alpha_L L$ (Strength)	0.84	1.55	1.40	1.70	0.96	1.74	1.74	2.07	1.09	1.94	2.10	2.48	1.22	2.14	2.49	2.92
	D+L (Deflection)	0.48	0.66	0.96	1.23	0.83	1.04	1.39	1.72	1.09	1.48	1.89	2.28	1.22	1.98	2.45	2.92
	L (Deflection)	0.48	0.66	0.96	1.23	0.83	1.04	1.39	1.72	1.09	1.48	1.89	2.28	1.22	1.98	2.45	2.92
6090	$\alpha_D D + \alpha_L L$ (Strength)	0.56	1.20	0.92	1.18	0.66	1.36	1.19	1.49	0.76	1.52	1.50	1.84	0.86	1.69	1.83	2.20
	D+L (Deflection)	0.10	0.25	0.49	0.72	0.37	0.55	0.84	1.12	0.70	0.90	1.25	1.58	0.86	1.31	1.71	2.11
	L (Deflection)	0.10	0.25	0.49	0.72	0.37	0.55	0.84	1.12	0.70	0.90	1.25	1.58	0.86	1.31	1.71	2.11
MAXIMUM UNSHORED CONSTRUCTION CLEAR SPANS (mm)																	
1span	2415	2785	3135	3370	2360	2720	3070	3305	2315	2665	3010	3240	2270	2610	2955	3185	
2span	2400	2775	3400	3845	2345	2705	3325	3770	2290	2650	3255	3695	2240	2590	3190	3620	
3span	2485	2870	3520	3975	2430	2805	3440	3895	2370	2740	3370	3820	2320	2680	3300	3745	
cantilever	885	1035	1320	1560	870	1020	1300	1535	865	1005	1275	1505	850	990	1255	1480	
Concrete Volume ( $m^3/m^2$ )	0.093				0.100				0.106				0.112				

2430	$\alpha_D D + \alpha_L L$ (Strength)	13.55
	D+L (Deflection)	13.55
	L (Deflection)	13.55

- Max. superimposed LSD factored dead + live load (kPa) (governed by strength limitation)
- Max. superimposed LSD unfactored dead + live load (kPa) (governed by deflection limitation of L/240)
- Max. superimposed LSD unfactored live load (kPa) (governed by deflection limitation of L/360)
- Vertical load span (center to center spacing)

- Wd Weight of deck (uncoated),  $kg/m^2$
- $I_D$  Moment of inertia for deflection per foot of deck width  $mm^4/m$
- Sp Section modulus for positive bending per foot of deck width,  $mm^3/m$
- Sn Section modulus for negative bending per foot of deck width,  $mm^3/m$
- f<sub>c</sub> 21 MPa
- $\alpha_D, \alpha_L$  Load factors for dead and live loads, respectively, to be applied by Engineer in accordance with Building Codes

- Rbe Allowable exterior web crippling value per foot of deck, kN/m
- Rbi Allowable interior web crippling value per foot of deck, kN/m
- h Total height of concrete slab, mm
- Wc Weight of concrete (neglecting deflection),  $kg/m^2$
- D Uniform dead load, kPa
- L Uniform live load, kPa

Construction spans shown based on 38 mm exterior bearing and 76 mm interior bearing width.

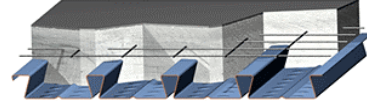
The section property table is based on 2001 AISI's Cold-Formed Steel Design Manual, 2004 Supplement. The live loads and unshored construction clear spans are based on the Canadian Sheet Steel Building Institute's Standard for Composite Steel Deck (CSSBI 12M-06), September 2006 and Criteria for the Design of Composite Slabs (CSSBI S3-2002), September 2003. The loads in these tables are based on a Simple Span Design Analysis.

2320  $KG/M^3$  CONCRETE

SECTION PROPERTIES

Fy = 276 MPa

GAGE	Wd	I <sub>D</sub>	Sp	Sn	Rbe			Rbi		
					51 mm	76 mm	102 mm	102 mm	127 mm	152 mm
22	10.90	555157	393153	383867	14.13	16.26	18.07	26.73	28.82	30.72
20	13.22	675466	493114	473586	20.11	23.06	25.54	37.95	40.82	43.43
18	17.48	897920	659170	661491	33.50	38.16	42.09	63.00	67.53	71.63
16	22.01	1136033	838335	843114	51.08	57.86	63.58	95.82	102.40	108.34



SIMPLE SPAN - MAXIMUM SUPERIMPOSED LSD LOADS, (kPa), NO STUDS ON BEAMS																	
h (Wc)		127 mm (275.72)				133 mm (290.47)				140 mm (305.21)				146 mm (319.96)			
Span (mm)	Load Combinations	GAGE															
		22	20	18	16	22	20	18	16	22	20	18	16	22	20	18	16
2430	$\alpha_D D + \alpha_L L$ (Strength)	18.61	20.47	23.94	23.94	19.91	20.52	23.94	23.94	21.22	20.41	23.94	23.94	22.54	23.94	23.94	23.94
	D+L (Deflection)	18.61	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
	L (Deflection)	18.61	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
2740	$\alpha_D D + \alpha_L L$ (Strength)	14.40	18.66	21.79	23.94	15.41	19.98	22.00	23.94	16.44	21.31	22.05	23.94	17.47	22.65	21.95	23.94
	D+L (Deflection)	14.40	18.66	19.15	19.15	15.41	19.15	19.15	19.15	16.44	19.15	19.15	19.15	17.47	19.15	19.15	19.15
	L (Deflection)	14.40	18.66	19.15	19.15	15.41	19.15	19.15	19.15	16.44	19.15	19.15	19.15	17.47	19.15	19.15	19.15
3040	$\alpha_D D + \alpha_L L$ (Strength)	11.37	15.00	22.25	22.02	12.18	16.06	23.94	22.31	13.00	17.14	23.94	23.94	13.82	18.23	23.94	23.94
	D+L (Deflection)	11.37	15.00	19.15	19.15	12.18	16.06	19.15	19.15	13.00	17.14	19.15	19.15	13.82	18.23	19.15	19.15
	L (Deflection)	11.37	15.00	19.15	19.15	12.18	16.06	19.15	19.15	13.00	17.14	19.15	19.15	13.82	18.23	19.15	19.15
3350	$\alpha_D D + \alpha_L L$ (Strength)	9.12	12.26	17.61	19.17	9.78	13.14	19.40	21.11	10.44	14.03	21.28	23.15	11.11	14.92	23.23	23.94
	D+L (Deflection)	9.12	12.26	17.61	19.15	9.78	13.14	19.15	19.15	10.44	14.03	19.15	19.15	11.11	14.92	19.15	19.15
	L (Deflection)	9.12	12.26	17.61	19.15	9.78	13.14	19.15	19.15	10.44	14.03	19.15	19.15	11.11	14.92	19.15	19.15
3650	$\alpha_D D + \alpha_L L$ (Strength)	7.40	10.16	14.08	15.38	7.94	10.89	15.55	16.98	8.48	11.63	17.09	18.65	9.03	12.38	18.69	20.40
	D+L (Deflection)	7.40	10.16	14.08	15.38	7.94	10.89	15.55	16.98	8.48	11.63	17.09	18.65	9.03	12.38	18.69	19.15
	L (Deflection)	7.40	10.16	14.08	15.38	7.94	10.89	15.55	16.98	8.48	11.63	17.09	18.65	9.03	12.38	18.69	19.15
3960	$\alpha_D D + \alpha_L L$ (Strength)	6.05	8.33	11.33	12.43	6.50	8.94	12.55	13.76	6.96	9.55	13.83	15.15	7.41	10.17	15.16	16.61
	D+L (Deflection)	6.05	8.33	11.33	12.43	6.50	8.94	12.55	13.76	6.96	9.55	13.83	15.15	7.41	10.17	15.16	16.61
	L (Deflection)	6.05	8.33	11.33	12.43	6.50	8.94	12.55	13.76	6.96	9.55	13.83	15.15	7.41	10.17	15.16	16.61
4260	$\alpha_D D + \alpha_L L$ (Strength)	4.98	6.87	9.15	10.09	5.36	7.38	10.17	11.20	5.32	7.90	11.24	12.37	5.68	8.42	12.36	13.60
	D+L (Deflection)	4.98	6.87	9.15	10.09	5.36	7.38	10.17	11.20	5.32	7.90	11.24	12.37	5.68	8.42	12.36	13.60
	L (Deflection)	4.98	6.87	9.15	10.09	5.36	7.38	10.17	11.20	5.32	7.90	11.24	12.37	5.68	8.42	12.36	13.60
4570	$\alpha_D D + \alpha_L L$ (Strength)	3.72	5.70	7.39	8.20	4.02	6.13	8.25	9.14	4.31	6.57	9.16	10.13	4.62	7.01	10.10	11.17
	D+L (Deflection)	3.72	5.70	7.39	8.20	4.02	6.13	8.25	9.14	4.31	6.57	9.16	10.13	4.62	7.01	10.10	11.17
	L (Deflection)	3.72	5.70	7.39	8.20	4.02	6.13	8.25	9.14	4.31	6.57	9.16	10.13	4.62	7.01	10.10	11.17
4870	$\alpha_D D + \alpha_L L$ (Strength)	2.96	4.74	5.95	6.65	3.20	5.11	6.68	7.45	3.45	5.05	7.45	8.30	3.70	5.41	8.25	9.18
	D+L (Deflection)	2.96	4.74	5.95	6.65	3.20	5.11	6.68	7.45	3.45	5.05	7.45	8.30	3.70	5.41	8.25	9.18
	L (Deflection)	2.96	4.74	5.95	6.65	3.20	5.11	6.68	7.45	3.45	5.05	7.45	8.30	3.70	5.41	8.25	9.18
5180	$\alpha_D D + \alpha_L L$ (Strength)	2.33	3.57	4.76	5.37	2.53	3.86	5.38	6.06	2.73	4.15	6.03	6.78	2.94	4.45	6.72	7.54
	D+L (Deflection)	2.33	3.57	4.76	5.37	2.53	3.86	5.38	6.06	2.73	4.15	6.03	6.78	2.94	4.45	6.72	7.54
	L (Deflection)	2.33	3.57	4.76	5.37	2.53	3.86	5.38	6.06	2.73	4.15	6.03	6.78	2.94	4.45	6.72	7.54
5480	$\alpha_D D + \alpha_L L$ (Strength)	1.80	2.90	3.76	4.30	1.96	3.15	4.29	4.88	2.13	3.39	4.85	5.50	2.30	3.64	5.44	6.16
	D+L (Deflection)	1.80	2.90	3.76	4.30	1.96	3.15	4.29	4.88	2.13	3.39	4.85	5.50	2.30	3.64	5.44	6.16
	L (Deflection)	1.80	2.90	3.76	4.30	1.96	3.15	4.29	4.88	2.13	3.39	4.85	5.50	2.30	3.64	5.44	6.16
5790	$\alpha_D D + \alpha_L L$ (Strength)	1.35	2.34	2.92	3.39	1.48	2.54	3.37	3.89	1.62	2.75	3.84	4.43	1.76	2.96	4.35	4.99
	D+L (Deflection)	1.35	2.34	2.92	3.39	1.48	2.54	3.37	3.89	1.62	2.75	3.84	4.43	1.76	2.96	4.35	4.99
	L (Deflection)	1.35	2.34	2.92	3.39	1.48	2.54	3.37	3.89	1.62	2.75	3.84	4.43	1.76	2.96	4.35	4.99
6090	$\alpha_D D + \alpha_L L$ (Strength)	0.97	1.86	2.19	2.62	1.07	2.03	2.58	3.05	1.18	2.20	4.06	3.51	1.30	2.38	4.55	3.99
	D+L (Deflection)	0.97	1.77	2.19	2.62	1.07	2.03	2.58	3.05	1.18	2.20	3.47	3.51	1.30	2.38	4.19	3.99
	L (Deflection)	0.97	1.77	2.19	2.62	1.07	2.03	2.58	3.05	1.18	2.20	3.47	3.51	1.30	2.38	4.19	3.99
MAXIMUM UNSHORED CONSTRUCTION CLEAR SPANS (mm)																	
1span	2225	2560	2910	3130	2185	2510	2860	3080	2145	2465	2815	3035	2110	2425	2775	2990	
2span	2190	2535	3125	3565	2145	2485	3065	3495	2100	2435	3010	3440	2055	2390	2955	3390	
3span	2270	2625	3235	3685	2220	2575	3170	3610	2175	2520	3110	3555	2130	2475	3055	3505	
cantilever	840	980	1235	1455	830	965	1220	1435	815	955	1200	1410	810	940	1185	1395	
Concrete Volume (m <sup>3</sup> /m <sup>2</sup> )	0.119				0.125				0.131				0.138				

2430	$\alpha_D D + \alpha_L L$ (Strength)	18.61	← Max. superimposed LSD factored dead + live load (kPa) (governed by strength limitation)
	D+L (Deflection)	18.61	← Max. superimposed LSD unfactored dead + live load (kPa) (governed by deflection limitation of L/240)
	L (Deflection)	18.61	← Max. superimposed LSD unfactored live load (kPa) (governed by deflection limitation of L/360)
↑ Vertical load span (center to center spacing)			
Wd	Weight of deck (uncoated), kg/m <sup>2</sup>		
I <sub>D</sub>	Moment of inertia for deflection per foot of deck width mm <sup>4</sup> /m		Rbe Allowable exterior web crippling value per foot of deck, kN/m
Sp	Section modulus for positive bending per foot of deck width, mm <sup>3</sup> /m		Rbi Allowable interior web crippling value per foot of deck, kN/m
Sn	Section modulus for negative bending per foot of deck width, mm <sup>3</sup> /m		h Total height of concrete slab, mm
f <sub>c</sub>	21 MPa		Wc Weight of concrete (neglecting deflection), kg/m <sup>2</sup>
$\alpha_D, \alpha_L$	Load factors for dead and live loads, respectively, to be applied by Engineer in accordance with Building Codes		D Uniform dead load, kPa
	Construction spans shown based on 38 mm exterior bearing and 76 mm interior bearing width.		L Uniform live load, kPa

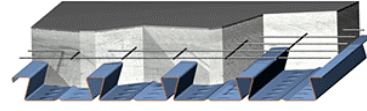
The section property table is based on 2001 AISI's Cold-Formed Steel Design Manual, 2004 Supplement. The live loads and unshored construction clear spans are based on the Canadian Sheet Steel Building Institute's Standard for Composite Steel Deck (CSSBI 12M-06), September 2006 and Criteria for the Design of Composite Slabs (CSSBI S3-2002), September 2003. The loads in these tables are based on a Simple Span Design Analysis.

2320 KG/M<sup>3</sup> CONCRETE

SECTION PROPERTIES

Fy = 276 MPa

GAGE	Wd	I <sub>D</sub>	Sp	Sn	Rbe			Rbi		
					51 mm	76 mm	102 mm	102 mm	127 mm	152 mm
22	10.90	555157	393153	383867	14.13	16.26	18.07	26.73	28.82	30.72
20	13.22	675466	493114	473586	20.11	23.06	25.54	37.95	40.82	43.43
18	17.48	897920	659170	661491	33.50	38.16	42.09	63.00	67.53	71.63
16	22.01	1136033	838335	843114	51.08	57.86	63.58	95.82	102.40	108.34



SIMPLE SPAN - MAXIMUM SUPERIMPOSED LSD LOADS, (kPa), NO STUDS ON BEAMS																	
h (Wc)		152 mm (334.71)				159 mm (349.46)				165 mm (364.21)				171 mm (378.96)			
Span (mm)	Load Combinations	GAGE															
		22	20	18	16	22	20	18	16	22	20	18	16	22	20	18	16
2430	$\alpha_D D + \alpha_L L$ (Strength)	23.88	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94	23.94
	D+L (Deflection)	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
	L (Deflection)	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
2740	$\alpha_D D + \alpha_L L$ (Strength)	18.51	23.94	23.94	23.94	19.55	23.94	23.94	23.94	20.60	23.94	23.94	23.94	21.66	23.94	23.94	23.94
	D+L (Deflection)	18.51	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
	L (Deflection)	18.51	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15	19.15
3040	$\alpha_D D + \alpha_L L$ (Strength)	14.65	19.32	23.94	23.94	15.48	20.42	23.94	23.94	16.32	21.53	23.94	23.94	17.16	22.64	23.94	23.94
	D+L (Deflection)	14.65	19.15	19.15	19.15	15.48	19.15	19.15	19.15	16.32	19.15	19.15	19.15	17.16	19.15	19.15	19.15
	L (Deflection)	14.65	19.15	19.15	19.15	15.48	19.15	19.15	19.15	16.32	19.15	19.15	19.15	17.16	19.15	19.15	19.15
3350	$\alpha_D D + \alpha_L L$ (Strength)	11.78	15.82	23.94	23.94	12.46	16.73	23.94	23.94	13.14	17.64	23.94	23.94	13.82	18.56	23.94	23.94
	D+L (Deflection)	11.78	15.82	19.15	19.15	12.46	16.73	19.15	19.15	13.14	17.64	19.15	19.15	13.82	18.56	19.15	19.15
	L (Deflection)	11.78	15.82	19.15	19.15	12.46	16.73	19.15	19.15	13.14	17.64	19.15	19.15	13.82	18.56	19.15	19.15
3650	$\alpha_D D + \alpha_L L$ (Strength)	9.59	13.14	20.34	22.22	10.14	13.89	21.51	23.94	10.70	14.66	22.69	23.94	11.26	15.42	23.88	23.94
	D+L (Deflection)	9.59	13.14	19.15	19.15	10.14	13.89	19.15	19.15	10.70	14.66	19.15	19.15	11.26	15.42	19.15	19.15
	L (Deflection)	9.59	13.14	19.15	19.15	10.14	13.89	19.15	19.15	10.70	14.66	19.15	19.15	11.26	15.42	19.15	19.15
3960	$\alpha_D D + \alpha_L L$ (Strength)	7.87	10.80	16.55	18.13	8.33	11.43	17.91	19.70	8.30	12.06	18.90	21.34	8.75	12.70	19.89	23.03
	D+L (Deflection)	7.87	10.80	16.55	18.13	8.33	11.43	17.91	19.15	8.30	12.06	18.90	19.15	8.75	12.70	19.15	19.15
	L (Deflection)	7.87	10.80	16.55	18.13	8.33	11.43	17.91	19.15	8.30	12.06	18.90	19.15	8.75	12.70	19.15	19.15
4260	$\alpha_D D + \alpha_L L$ (Strength)	6.05	8.94	13.53	14.88	6.42	9.47	14.74	16.20	6.79	10.00	15.89	17.58	7.16	10.53	16.74	19.01
	D+L (Deflection)	6.05	8.94	13.53	14.88	6.42	9.47	14.74	16.20	6.79	10.00	15.89	17.58	7.16	10.53	16.74	19.01
	L (Deflection)	6.05	8.94	13.53	14.88	6.42	9.47	14.74	16.20	6.79	10.00	15.89	17.58	7.16	10.53	16.74	19.01
4570	$\alpha_D D + \alpha_L L$ (Strength)	4.92	7.45	11.09	12.25	5.23	7.89	12.11	13.38	5.54	7.84	13.18	14.55	5.85	8.27	14.19	15.77
	D+L (Deflection)	4.92	7.45	11.09	12.25	5.23	7.89	12.11	13.38	5.54	7.84	13.18	14.55	5.85	8.27	14.19	15.77
	L (Deflection)	4.92	7.45	11.09	12.25	5.23	7.89	12.11	13.38	5.54	7.84	13.18	14.55	5.85	8.27	14.19	15.77
4870	$\alpha_D D + \alpha_L L$ (Strength)	3.95	5.76	9.09	10.11	4.20	6.12	9.97	11.07	4.46	6.48	10.88	12.07	4.72	6.84	11.82	13.11
	D+L (Deflection)	3.95	5.76	9.09	10.11	4.20	6.12	9.97	11.07	4.46	6.48	10.88	12.07	4.72	6.84	11.82	13.11
	L (Deflection)	3.95	5.76	9.09	10.11	4.20	6.12	9.97	11.07	4.46	6.48	10.88	12.07	4.72	6.84	11.82	13.11
5180	$\alpha_D D + \alpha_L L$ (Strength)	3.14	4.75	7.44	8.33	3.35	5.05	8.19	9.16	3.56	5.35	8.97	10.02	3.78	5.66	9.77	10.91
	D+L (Deflection)	3.14	4.75	7.44	8.33	3.35	5.05	8.19	9.16	3.56	5.35	8.97	10.02	3.78	5.66	9.77	10.91
	L (Deflection)	3.14	4.75	7.44	8.33	3.35	5.05	8.19	9.16	3.56	5.35	8.97	10.02	3.78	5.66	9.77	10.91
5480	$\alpha_D D + \alpha_L L$ (Strength)	2.47	3.90	6.05	6.84	2.64	4.15	6.69	7.55	2.82	4.41	7.37	8.30	2.99	4.67	8.06	9.07
	D+L (Deflection)	2.47	3.90	6.05	6.84	2.64	4.15	6.69	7.55	2.82	4.41	7.37	8.30	2.99	4.67	8.06	9.07
	L (Deflection)	2.47	3.90	6.05	6.84	2.64	4.15	6.69	7.55	2.82	4.41	7.37	8.30	2.99	4.67	8.06	9.07
5790	$\alpha_D D + \alpha_L L$ (Strength)	1.90	3.18	4.88	5.58	2.04	3.39	6.40	6.20	2.18	3.61	6.79	6.84	2.32	3.83	7.17	7.51
	D+L (Deflection)	1.90	3.18	4.88	5.58	2.04	3.39	6.40	6.20	2.18	3.61	6.79	6.84	2.32	3.83	7.17	7.51
	L (Deflection)	1.90	3.18	4.88	5.58	2.04	3.39	6.40	6.20	2.18	3.61	6.79	6.84	2.32	3.83	7.17	7.51
6090	$\alpha_D D + \alpha_L L$ (Strength)	1.41	2.56	5.05	4.50	1.52	2.74	5.46	5.04	1.32	2.93	5.79	5.60	1.43	3.11	6.13	6.18
	D+L (Deflection)	1.41	2.56	4.97	4.50	1.52	2.74	5.46	5.04	1.32	2.93	5.79	5.60	1.43	3.11	6.13	6.18
	L (Deflection)	1.41	2.56	4.97	4.50	1.52	2.74	5.46	5.04	1.32	2.93	5.79	5.60	1.43	3.11	6.13	6.18
MAXIMUM UNSHORED CONSTRUCTION CLEAR SPANS (mm)																	
1span	2075	2385	2735	2945	2045	2345	2695	2910	2015	2310	2660	2870	1985	2275	2625	2835	
2span	2020	2345	2900	3340	1980	2305	2855	3290	1945	2265	2805	3240	1910	2225	2760	3190	
3span	2090	2430	3000	3455	2050	2385	2950	3400	2015	2345	2900	3350	1980	2305	2855	3300	
cantilever	800	930	1170	1370	790	915	1155	1355	785	910	1140	1340	775	895	1130	1320	
Concrete Volume (m <sup>3</sup> /m <sup>2</sup> )	0.144				0.150				0.157				0.163				

2430	$\alpha_D D + \alpha_L L$ (Strength)	23.88	← Max. superimposed LSD factored dead + live load (kPa) (governed by strength limitation)
	D+L (Deflection)	19.15	← Max. superimposed LSD unfactored dead + live load (kPa) (governed by deflection limitation of L/240)
	L (Deflection)	19.15	← Max. superimposed LSD unfactored live load (kPa) (governed by deflection limitation of L/360)
Vertical load span (center to center spacing)			
Wd	Weight of deck (uncoated), kg/m <sup>2</sup>		
I <sub>D</sub>	Moment of inertia for deflection per foot of deck width mm <sup>4</sup> /m		Rbe Allowable exterior web crippling value per foot of deck, kN/m
Sp	Section modulus for positive bending per foot of deck width, mm <sup>3</sup> /m		Rbi Allowable interior web crippling value per foot of deck, kN/m
Sn	Section modulus for negative bending per foot of deck width, mm <sup>3</sup> /m		h Total height of concrete slab, mm
f <sub>c</sub>	21 MPa		Wc Weight of concrete (neglecting deflection), kg/m <sup>2</sup>
$\alpha_D, \alpha_L$	Load factors for dead and live loads, respectively, to be applied by Engineer in accordance with Building Codes		D Uniform dead load, kPa
	Construction spans shown based on 38 mm exterior bearing and 76 mm interior bearing width.		L Uniform live load, kPa

The section property table is based on 2001 AISI's Cold-Formed Steel Design Manual, 2004 Supplement. The live loads and unshored construction clear spans are based on the Canadian Sheet Steel Building Institute's Standard for Composite Steel Deck (CSSBI 12M-06), September 2006 and Criteria for the Design of Composite Slabs (CSSBI S3-2002), September 2003. The loads in these tables are based on a Simple Span Design Analysis.

2320 KG/M<sup>3</sup> CONCRETE